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# Comparative Numerical Study on Focusing Wave Interaction with FPSO-like Structure

Shiqiang Yan<sup>1</sup>, Zhihua Xie<sup>2\*</sup>, Qian Li<sup>1</sup>, Jinghua Wang<sup>1</sup>, Qingwei Ma<sup>1</sup>, Thorsten Stoesser<sup>3</sup>

1: School of mathematics, Computer science and engineering, City, University of London, London, UK

2: School of Engineering, Cardiff University, Cardiff, UK

3: Department of Civil, Environmental and Geomatic Engineering, University College London, London, UK

\*Corresponding author: zxie@cardiff.ac.uk

## ABSTRACT

**Evaluating the interactions** between offshore structures and extreme waves plays an essential role for securing the survivability of the structures. For this purpose, various numerical tools, e.g. the fully nonlinear potential theory (FNPT), the Navier-Stokes (NS) models and hybrid approaches combining different numerical models, have been developed and utilised. However, there is still a great uncertainty over the required level of model fidelity when being applied to a wide range of wave-structure interaction problems. This paper aims to shed some light on this issue with a specific focus on the overall error sourced from wave generation/absorbing techniques and resolving the viscous and turbulent effects, by comparing the performances of three different models, including the quasi Arbitrary Lagrangian Eulerian Finite Element Method (QALE-FEM) based on the FNPT, an in-house two-phase NS model with large-eddy simulation (LES) and a hybrid model coupling the QALE-FEM with the OpenFOAM/InterDymFoam, in the cases with a fixed FPSO-like structure in extreme focusing waves. The relative errors of numerical models are defined against the experimental data, which are released after the numerical works have been completed (i.e. a blind test), in terms of the pressure and wave elevation. It provides a practical reference for not only choosing an appropriate model in practices but also on developing/optimizing numerical tools for more reliable and robust predications.

**KEY WORDS:** Wave-structure interaction, comparative study, blind test, FNPT, NS models, LES.

## INTRODUCTION

Understanding the characteristics of the interaction between the extreme waves and structures, as well as a reliable prediction of the behavior of the structures in a realistic extreme sea, plays a fundamental role in the safe and cost-effective design of coastal and offshore structures, and marine renewable devices. Such assessments and predictions can always be performed in a laboratory environment or in a numerical wave tank, where the extreme waves are often modeled by using a focusing wave based on the spatial-temporal focusing mechanism (Ma, *et al.*, 2015) or the NewWave theory (Tromans, *et al.*, 1991).

In design practices, classical approaches in frequency domain are employed, such as linear and second order theories, which however are shown to be insufficient when higher-order nonlinear effects are pronounced. Such higher-order nonlinearities have been pointed out to play important roles in the interactions between extreme waves and structures (Zang, *et al.*, 2010). To overcome this drawback, approaches in time domain with considering sufficient nonlinearities have been developed. These include the fully nonlinear potential theory (FNPT) and the general viscous flow theory (NS), which is formulated by the Navier-Stokes equations. The FNPT model, which

assumes that the flow is inviscid and irrotational, is solved by different numerical methods, such as the boundary element method (e.g. Longuet-Higgins & Cokelet, 1976; Grilli, *et al.* 2001), finite element method (e.g. Wu & Eatock-Taylor, 2003; Yan & Ma, 2007; Ma & Yan, 2009), spectral element method (e.g. Engsig-Karup, *et al.* 2016). The NS model does not have the assumptions of the FNPT model. However, in the real application of the NS model to the wave-structure interactions, certain degrees of simplification might be applied, resulting different forms of the NS models with main diversities including (1) one-phase (Ma 2005; Zhou and Ma, 2010; Lind *et al.*, 2012; Zheng *et al.*, 2014) or two-phase (Chen *et al.*, 2014; Hildebrandt and Sriram, 2014; Ferrer *et al.*, 2016; Hu *et al.*, 2016; Xie, 2012); (2) compressible (Yang *et al.*, 2016) or incompressible (Yang *et al.*, 2017). The significant diversities also exist in terms of numerical approaches on (1) how to solve the governing equation, for which both the conventional mesh-based methods, e.g. the finite volume method (Chen, *et al.*, 2014; Hildebrandt & Sriram, 2014; Xie, 2015; Yang *et al.*, 2017) or meshless methods, e.g. the smoothed particle hydrodynamics (SPH, e.g. Lind, *et al.*, 2012; Zheng, *et al.*, 2014) and the meshless local Petrov-Galerkin (MLPG\_R) method (Ma, 2005; Zhou & Ma, 2010; Sriram & Ma, 2012), have been attempted; (2) how to track or capture the free surface, e.g. the volume of fluid (VOF, e.g. Hu *et al.*, 2016; Xie, 2012) and level set method (Zhang *et al.*, 2009; Yang *et al.*, 2017); and (3) how to model the turbulence, e.g. the Reynolds Averaged NS (Yang *et al.*, 2016) or Large-eddy simulation (LES, e.g. Xie, 2015).

Generally speaking, the computational demands of FNPT models are considerably lower than those of NS models. The accuracy and reliability of the FNPT models on simulating extreme wave evolutions have been widely recognized (e.g. Yan and Ma, 2010b; Engsig-Karup, *et al.* 2016; Wang *et al.*, 2018). However, the FNPT models cannot reliably capture the small-scale viscous and turbulent effects, which may be significant near the structures, e.g. the slamming and breaking wave impact on structures (Dias and Ghidaglia, 2018), wave motions in a small region confined by structures (e.g. Lu and Chen, 2012), due to their theoretical assumption. This initiates the developments of hybrid approaches combining the FNPT or other simplified models with a NS model for robustly modelling the wave-structure interactions. The fundamental idea of the hybrid approach is that in the regions where the viscous/turbulent effects are significant, e.g., near the breaking waves and the structures, the NS model is utilised to resolve small- and micro-scale physics, e.g. the vortex shedding and flow separation; in other regions, e.g. the wave propagation away from the structures, the FNPT model or other simplified models are employed. Models based on the velocity-decomposition (e.g. Ferrant *et al.*, 2003; Luquet *et al.*, 2007, Ferrant *et al.*, 2008; Edmund *et al.*, 2013) and the domain-decomposition (e.g. Yan and Ma 2010; Hildebrandt *et al.*, 2013; Sriram *et al.*, 2014; Fourtakas *et al.*, 2017; Yan and Ma, 2017; Higuera *et al.*, 2018; Li & Wang *et al.*, 2018) have been developed. Systematic reviews on the development of the hybrid models can be found in Sriram *et al.*, (2014) and Li & Wang *et al.* (2018).

Comparative studies have been carried out to investigate the performances of different models for specific problems aiming to provide a reference or guideline for the selection of suitable numerical models, e.g. on the water entry problem (Hong *et al.*, 2017) and slamming impact (Dias and Ghidaglia, 2018). It has found that different numerical tools (theories) or the same numerical tool run by different persons may produce considerably different results (e.g. Hong *et al.*, 2017). This implies that there is still a great uncertainty over the required level of model fidelity when being applied to a wide range of wave-structure interactions. Such uncertainty may be sourced from effectiveness of generating incoming waves, physical simplification, numerical truncation error and discretization. This paper focuses on a quantitative assessment of the overall performance of different models. The problem to be considered is a focused wave interaction with a fixed FPSO-like structures. The wave conditions, FPSO model, arrangements of the pressure sensors and wave gauges are pre-specified by the CCP-WSI consortium ([www.ccp-wsi.ac.uk](http://www.ccp-wsi.ac.uk)) through its blind test workshops. Three numerical methods including the QALE-FEM based on the FNPT (Ma and Yan, 2009), a two-phase NS model with LES (Xie, 2015) and a hybrid model coupling the QALE-FEM with OpenFOAM/InterDymFoam (Li & Wang *et al.*, 2018). In order to realistically reflect the overall performance of different numerical models in practices without artificial numerical calibration or data smoothing, the comparison was carried out using a blind-test approach, in which the experimental data were released after the numerical simulations have been completed. However, it causes diversities in numerical practices, e.g. the wave generation and absorption, the computational domain sizes, the fluid properties and computing hardware.

## NUMERICAL METHODS

The QALE-FEM method is based on the FNPT. The velocity potential and its time derivative, which is required by the Bernoulli's equation for the pressure evaluation, are governed by the corresponding Laplace's equations and solved by boundary value problems, where

the free surface boundary condition is written in an arbitrary Lagrangian-Eulerian form (Li & Wang *et al.*, 2018), using the FEM. The details of the FEM formulations and the time integration scheme have been discussed in our previous publications, e.g. Ma *et al.* (2001). The main differences between the QALE-FEM method and the conventional FEM method is that the computational mesh in the QALE-FEM is moving by using a novel methodology based on the spring analogy method but purpose-developed for wave-structure interaction problems. The details of the QALE-FEM are referred to Ma and Yan (2009) and Yan and Ma (2007).

The two-phase NS model developed by Xie (2012, 2013, 2014, 2015 and 2017) employs the LES approach with the dynamic Smagorinsky sub-grid scale (SGS) model (Lily, 1992). The high-resolution VOF scheme CICSAM (Compressive Interface Capturing Scheme for Arbitrary Meshes) (Ubbink, 1997) is used for identifying the water/air phase and thus the free surface. The governing equations are discretised using the finite volume method (FVM) on a staggered Cartesian grid. The advection terms are discretised by a high-resolution scheme (Xie, 2012), which combines the high order accuracy with monotonicity, whereas the gradients in pressure and diffusion terms are obtained by central difference schemes. In order to deal with complex geometries in Cartesian grids, the partial cell treatment in 3D (Xie, 2015) was utilized in the finite volume discretisation. The SIMPLE algorithm (Patankar, 1980) is employed in for the pressure-velocity coupling and a backward finite difference discretisation is used for the time derivative. More details of this model can be found in Xie (2012, 2013, 2014, 2015 and 2017). The computer code is parallelized using MPI and a domain decomposition technique.

The hybrid FNPT-NS solver, qaleFOAM, combines the QALE-FEM and OpenFOAM using a zonal approach, in which the FNPT domain, governed by the QALE-FEM, covers the whole computational domain and the NS domain is a confined zone around the structure, governed by the multiphase NS solver OpenFOAM/interDyMFOam. On the coupling boundary, the velocity, pressure and wave elevation for the NS solver are provided by the QALE-FEM. A relaxation zone is applied near coupling boundaries to (1) absorb the reflected waves from the structures, and (2) ensure a smooth transition of the solution. It shall be noted that the solution in the NS domain does not feedback to the FNPT domain and the coupling of two models in this paper is a weak (one-way) coupling. More details of the qaleFOAM can be found in Li & Wang *et al.* (2018).

## EXPERIMENTS AND CASE CONFIGURATIONS

The experiments were performed in the wave basin at the University of Plymouth, which features 35 m in length, 15.5 m in width and 2.93 m in depth. Flap wave paddles are installed to generate three-dimensional waves. A simplified FPSO with two ends being semi-circles (0.15 m radius) is used in the experiments. The length, width and the height of the FPSO model is 1.2 m, 0.3 m and 0.3 m, respectively, as illustrated in Fig. 1. The temporal variations of surface elevation at various locations are recorded by wave gauges with a sampling frequency of 128 Hz. Those near or on the surface of the FPSO are illustrated in Fig. 1. Six pressure sensors are installed near the bow of the FPSO surface. Pressure sensors P1, P2 and P3 are placed at the bow and located at 0.05 m above mean free surface (MWL), in line with the MWL and 0.05 m below the MWL, respectively. The vertical spacing of the sensor groups P4-P6 are the same as P1-P3, but they are located at 45° about the longitudinal axis of the FPSO. More details of the arrangement of the gauges and the pressure sensors can be found in Ransley *et al.* (2018). The focusing

waves are generated by using flap wave paddles whose motions are specified by using the 2<sup>nd</sup> order wavemaker theory. JONSWAP spectrum with different significant wave heights ( $H_s$ ) and peak periods ( $T_p$ ) are considered in this study. The parameters describing the wave conditions, including the local wave steepness ( $kA$ ), are summarized in Table 1.

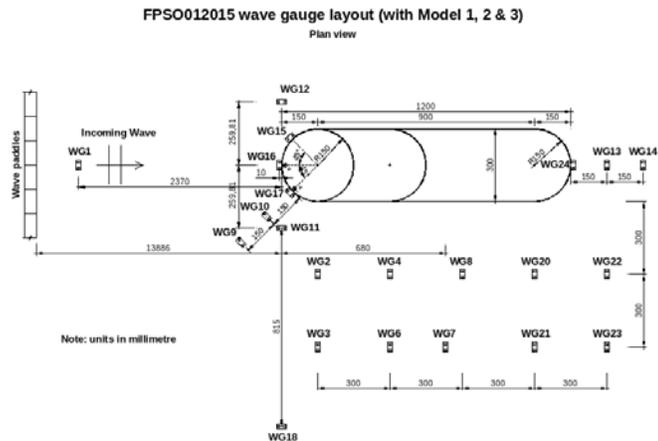


Fig. 1: Sketch of the experimental configuration (duplicated from [https://www.ccp-wsi.ac.uk/blind\\_test\\_series\\_1](https://www.ccp-wsi.ac.uk/blind_test_series_1))

Table 1. Wave parameters for each of the test cases

ID	$T_p$ (s)	$H_s$ (m)	$kA$
11BT1	1.456	0.077	0.13
12BT1	1.456	0.103	0.18
13BT1	1.362	0.103	0.21

## NUMERICAL RESULTS AND DISCUSSION

### Numerical configurations

The numerical wave tank used by the QALE-FEM has the same size as the experimental wave basin, i.e. 35m  $\times$  15.5m  $\times$  2.93m. To accelerate the simulation, the approach using the concept of the overset grid method (Ma *et al.* 2015) is applied. In this approach, two sets of computational mesh are used. One covers the entire computational domain without the FPSO for modelling the nonlinear incident waves. Since the incident wave is unidirectional, the mesh size in the direction normal to the wave propagation can be very coarse, e.g. 4 cells along the transverse direction of the tank. Another one covers a confined zone near the FPSO, i.e. a circular domain with diameter of 7 m centred at the geometric centre of the FPSO. At the outer boundary of the confined zone, the free surface elevation and the fluid velocity are specified by using the solutions from the first set of mesh. A translational zone with a width of 1 m is placed near the outer boundary of the confined zone, in order to damp the reflection from the FPSO.

The computational domain and the mesh resolution used in the FNPT domain of the qaleFOAM is the same as the first set of mesh in the QALE-FEM. The NS domain of the qaleFOAM has a size of 5.4 m  $\times$  3 m  $\times$  3.53 m, where the width of the relaxation zone attached to the outer boundary of the NS domain is 0.6 m. The FPSO is placed at the centre of the NS domain. A laminar model is used in the NS domain. Free-slip boundary conditions are imposed at the bottom, top and side walls of the wave tanks in order to avoid resolving the boundary layers.

The computational domain of the two-phase NS model with LES has a size of 6m  $\times$  3m  $\times$  3.3m. The structure is 2.37 m away from the inlet, which is exactly the distance between the wave gauge WG1 and the bow of the structure in the experiment. With this setup, the measurement at WG1 can be used directly to specify the inlet wave condition. On the structure boundary, the wall model is used for the near-wall treatment in the LES modelling. On other boundaries, the same boundary conditions as the NS domain of the qaleFOAM are utilised.

### Wave Generation and Absorption

In the QALE-FEM and the qaleFOAM, the wave is generated by using a wavemaker. Due to the fact that the wave paddle motions are unknown, it is necessary to find the wavemaker motion to reproduce the incident waves. To do so, the self-correction technique (Ma, *et al.*, 2015) is employed in this study. For all cases considered here, the wave elevation recorded by WG1 in the empty-tank test without the FPSO is used as the targeted wave to reproduce the wavemaker motion. After that, the same wavemaker motions are used to drive the wavemakers in the QALE-FEM and the qaleFOAM simulations. In the LES model, the wave is generated by specifying the inlet boundary condition in terms of the wave elevation, velocity and pressure, for which the wave elevation recorded at WG1, placed at the inlet of the LES domain, in the empty tank test is used to derive the wave spectrum and the linear wave theory is adopted to find the particle velocity and elevation at the inlet of the computational domain.

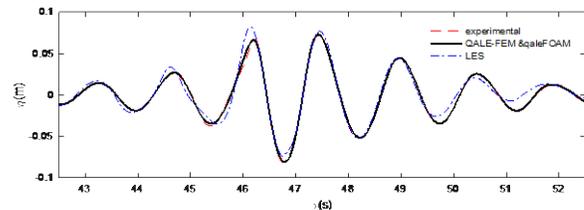


Fig. 2 Comparison of wave elevations recorded at WG1 in the empty-tank test of Case 12BT1

In order to check effectiveness of the wave generation, the time histories of the wave elevation at WG1 are compared with the experimental data (Fig. 2). Since the QALE-FEM and the qaleFOAM use the same wave generation technology, the results at WG1 are identical and agree well with the experimental data; whereas a noticeable discrepancy is observed between the LES results and the experimental data.

It is worth noting that the wave generation by specifying the inlet boundary conditions using the wave theory for steady wave, e.g. the linear and 2<sup>nd</sup> order wave theories, can result in a satisfactory result but is subjected to a technical constraint, i.e. the wave spectrum at the inlet must be known at prior. This means that the inlet boundary (wave generation boundary) must be placed at a location where the time history of the wave elevation is known and recorded by a wave gauge. If such a gauge position (as well as the inlet boundary of the computational domain) is too far away from the structures, the computational domain may become unnecessarily big, leading to a long CPU time; if it is too close to the structures, there might be an insufficient space between the inlet and the structures to accommodate a damping/relaxation zone for suppressing the reflection wave from the structures. On the contrary, the self-correction wavemaker technique does not have such constraint. Once the wavemaker motion is determined by a targeted wave, e.g. that at WG1, the FNPT model

in the QALE-FEM and the qaleFOAM can well capture the nonlinear wave evolutions, as demonstrated by Fig. 3, which compares the wave elevations at other positions, i.e. WG16 and WG24, using the QALE-FEM with the experimental data. This means that the boundary of the NS domain in the qaleFOAM can be placed at any positions, even though there are no measured data available. The QALE-FEM in the qaleFOAM can provide an accurate prediction of the wave kinematics at the inlet of the NS domain. This can be further evidenced by the satisfactory agreement between the qaleFOAM results with the experimental data in Fig. 3, in which the inlet of the NS domain is placed 2.1 m upstream to the bow of the FPSO. In other words, the coupling of the QALE-FEM and the NS solver in the qaleFOAM provides a robust way to explore both the large-scale nonlinear wave evolution and the small-scale physics such as the viscous and turbulent effects near the structures simultaneously (Li & Wang *et al.*, 2018).

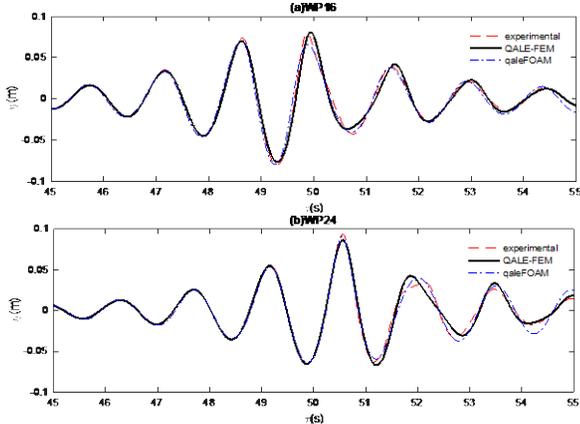


Fig. 3 Comparison of wave elevations recorded at WG16 and WG24 in the empty-tank test of Case 12BT1

In the QALE-FEM and qaleFOAM, a self-adaptive wavemaker is adopted in the right end of the numerical wave tank to absorb the wave, avoiding reflection from the outlet. In the LES model, the zero-gradient or radiation boundary conditions are applied for the flow at the outlet of the computational domain.

Table 2: Computational mesh in the NS domain of QALE-FEM

Mesh No	Mesh grid number	Number of grids on the structure surface
M1	2352996	958
M2	3449136	3576
M3	3797136	7576

### Extreme Wave Interaction with FPSO-like Structure

The main focus of this paper is to compare the performance of three models in the cases with FPSO. The results are discussed in this section.

#### Convergence

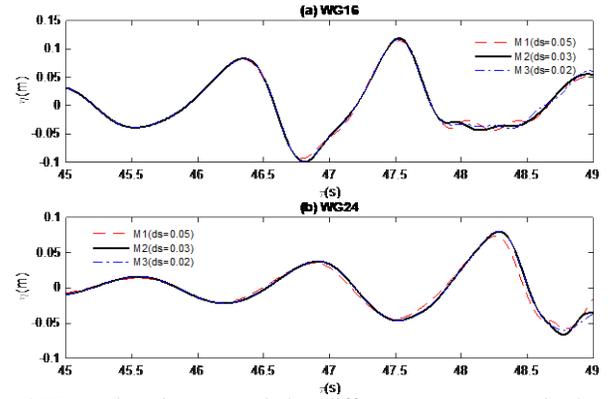


Fig. 4 Wave elevations recorded at different wave gauges in the cases with different mesh sizes using the QALE-FEM (Case 13BT1)

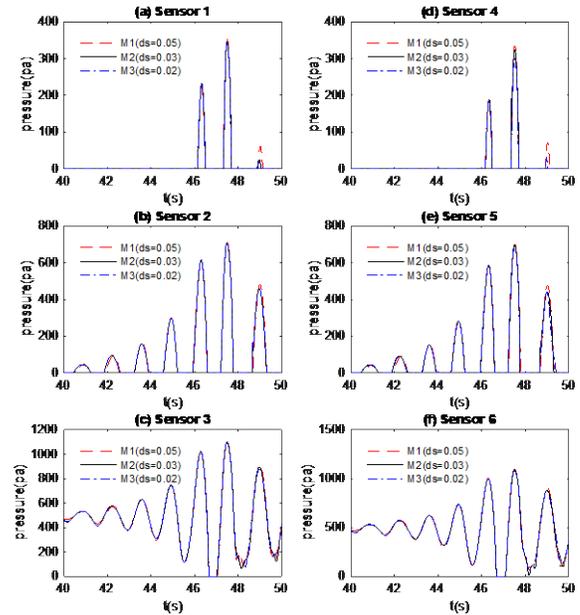


Fig. 5 Time histories of the pressure recorded at different locations in the cases with different mesh sizes using the QALE-FEM (Case 13BT1)

Convergence investigations are carried out using meshes with different resolutions. In the QALE-FEM simulation, the time step size adopted is the same as the experimental sampling time interval, i.e. 128 Hz. Three sets of the computational mesh, where the characterized mesh size on the free surface ranges from 0.02 to 0.05 m and the characterized mesh size near the structure surface ranges from 0.01 to 0.025 m. The mesh size exponentially increases from the free surface to the tank bottom. Table 2 summarises three sets of mesh used by the QALE-FEM. Some results obtained by the QALE-FEM are shown in Fig. 4 and Fig. 5 to shed lights on the convergence property of the QALE-FEM. These figures, respectively, illustrate the time histories of the wave elevation and pressure recorded at different locations for Case 13BT1, which involves the steepest wave in this paper. As observed, the corresponding results using M2 is similar to those adopting M3 (finest resolution) but different from those applying M1 (coarsest resolution). A similar phenomenon is also observed in other cases. This justifies that M2 is sufficient for the QALE-FEM to obtain the convergent solution.

The time step size used in both the QALE-FEM and the NS solver of

the qaleFOAM is dynamically updated by satisfying the Courant condition in the NS domain, i.e. the Courant number is smaller than 0.5, due to the fact that the time step required to obtain convergent results by the QALE-FEM is larger than that by the NS solver. In the FNPT domain of the qaleFOAM, the characterized mesh size on the free surface is approximately 0.03 m in the longitude direction for all cases presented here. Three set of computational mesh used in the NS domain of the qaleFOAM. Detailed results for the convergence of the qaleFOAM has been discussed in Li & Yan *et al* (2018) and the corresponding results will not be repeated here. The time step used in the LES modelling is uniform, i.e. 0.0001 s. The computational domain of the LES is discretized using a uniform grid of  $320 \times 320 \times 160$  points in the longitude (streamwise), vertical and transverse (spanwise) directions, respectively. The total number of cells is approximately 16.4 M. The choice of such mesh resolution is based on the experience in modelling the wave-structure interaction (Xie, 2015) and a further confirmation that the predictions using this mesh agree well with a corresponding result with a slightly coarser mesh.

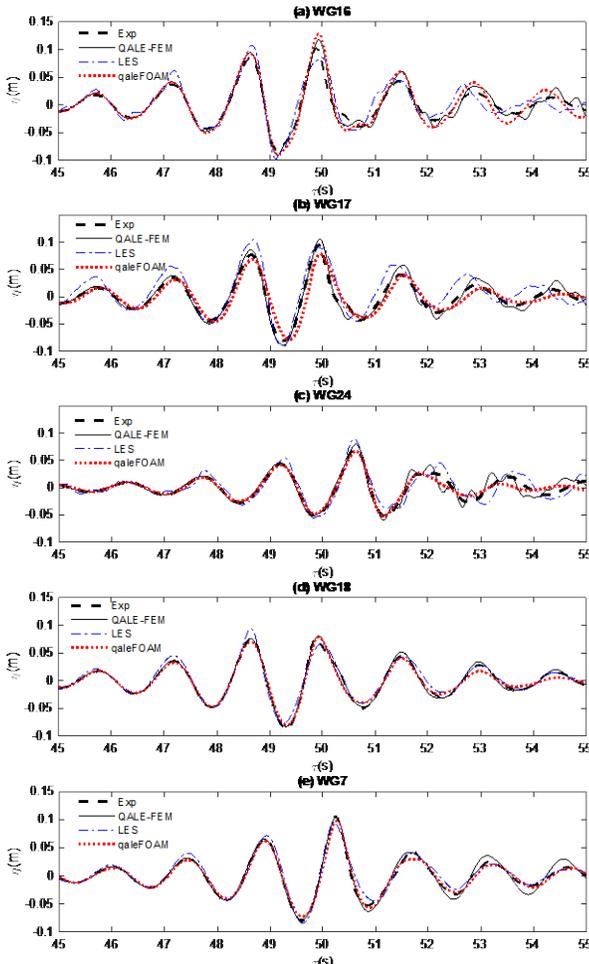


Fig. 6 Time histories of the wave elevation recorded at different positions (Case 12BT1)

### Overall Accuracy

In this section, the convergent results from the QALE-FEM, the LES and the qaleFOAM are compared with the experimental data in order to compare the overall performance of these models on numerically simulating the fixed FPSO in non-breaking extreme waves.

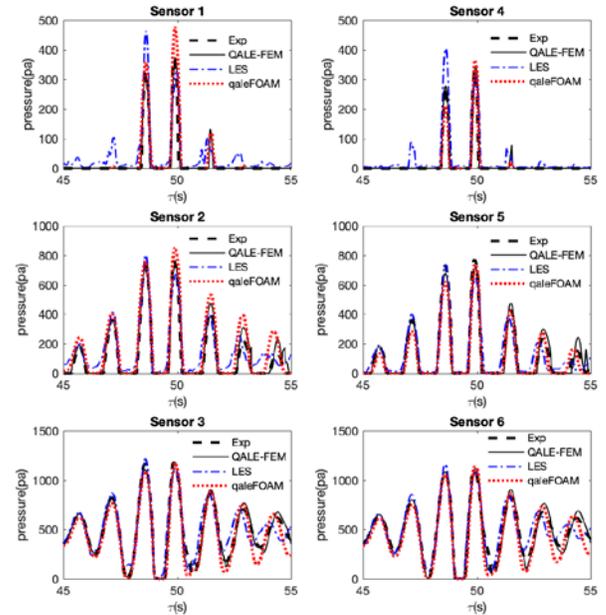
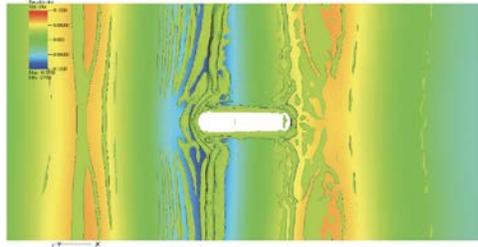


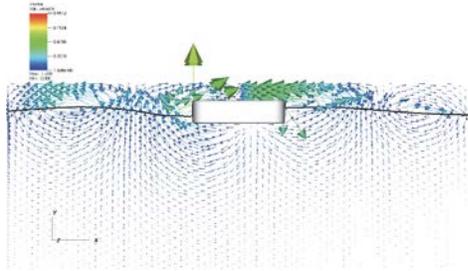
Fig. 7 Time histories of the pressure recorded at different positions on the FPSO surface(Case 12BT1)

The first case considered is Case 12BT1, where the focusing wave steepness  $kA = 0.18$ . Fig. 6 compares the wave elevations recorded at different positions near the FPSO. At the positions away from the FPSO, i.e. WG7 (Fig.6(e)) and WG18(Fig. 6(d)), the waves are less disturbed by the FPSO, and, therefore, the corresponding results can be used to evaluate whether the incoming waves are well reproduced by different wave generation approaches. From Fig. 6(d) and (e), it is observed that the numerical results by the QALE-FEM and the qaleFOAM, where the self-correction wavemaker technology is used, agree well with the experimental data; whereas the results obtained by the LES, in which a linear wave theory is used to specify the inlet boundary condition, show noticeable discrepancies with the experimental data, especially in terms of the occurrences of the highest wave crest in Fig. 6(d), as well as at the inlet (Fig. 2). For the LES, similar phenomenon is also found on the runup (Fig. 6 (a & b)) and the pressure on the bow of FPSO (e.g. Sensors 1 and 4 in Fig. 7). This suggests that the effective wave generation must be considered as a critical issue when modelling wave-structure interaction, especially in the cases with extreme waves.

One may also find from Fig. 6 and Fig. 7 that the accuracies of the FNPT model (QALE-FEM), where the viscous effect is ignored, and that the NS models are at a similar level, suggesting that the viscous and turbulent effects may be insignificant. This can be further demonstrated by Fig. 8, which displays the vortical structure and the velocity field near the FPSO when the focusing trough occurs at the bow of the FPSO. As observed from Fig. 8(a), the presence of the FPSO mainly disturb the vorticity near the surface of the FPSO. The magnitude of the vorticity near the FPSO seem to be very similar to those in the far-field wave region and no clear vortex shedding is observed (Fig. 8(b)).



(a) Vortical structure during focusing wave impact



(b) Velocity field near the FPSO

Fig.8 Vortical structure and velocity field near the FPSO (Case 12BT1, the results are obtained by the LES)

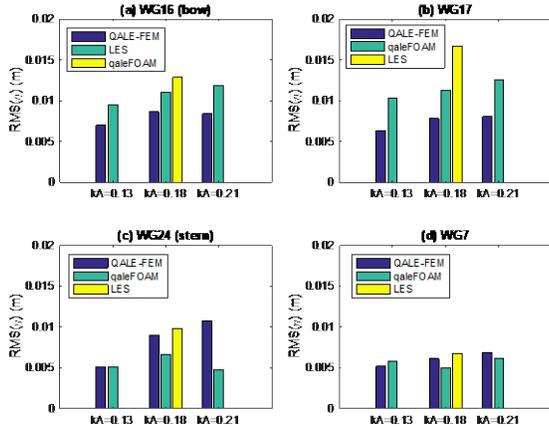


Fig.9 RMS of the wave run-up in the cases with different wave steepness

In practices, the wave runup on the floating structure (e.g. WG16, WG17 and WG24), the diffraction wave near the FPSO (e.g. WG7) and the maximum pressure acting on the structure surface receive the most concern. In order to quantify the accuracy of numerical models, two types of errors are defined as below,

$$RMS(\eta) = \sqrt{\frac{\sum_{i=1}^N (\eta_n - \eta_e)^2}{N}} \quad (1)$$

$$\epsilon(p^{peak}) = (p_n^{peak} - p_e^{peak}) / p_e^{peak} \quad (2)$$

where the subscripts  $n$  and  $e$  denote the numerical and experimental data, respectively;  $\eta$  the time history of the wave runup;  $p^{peak}$  is the peak pressure. The time histories of the wave elevation in a duration of 10s centred at the instant when the maximum wave elevation occurs are used to evaluate the RMS of the wave elevation using Eq. (1). The corresponding errors in the cases with different wave steepness are compared and the results are shown in Fig. 9 and Fig. 10 for the wave runup and the peak pressure on the FPSO surface, respectively.

It is observed from Fig. 9(d) that  $RMS(\eta)$  for all numerical methods are similar for the wave elevation recorded at WG7, which is located away from the FPSO (Fig. 1). It is found from Fig. 9(c) that, on or near the bow of the FPSO (WG16, WG17),  $RMS(\eta)$  for the QALE-FEM is slightly smaller than others; whereas, on the stern of the FPSO (WG24), such superiority of the QALE-FEM disappears, partially due to the fact that the boundary layer separation likely occurs and consequently more significant turbulent effects are expected. At all gauge positions presented in Fig. 9,  $RMS(\eta)$  of the QALE-FEM increases as the increase of the wave steepness. From Fig. 10, one may find that  $\epsilon(p^{peak})$  of the QALE-FEM and the LES is typically lower than 10% (Fig. 10), however, that of the qaleFOAM is significantly larger for the peak pressure recorded by the pressure sensors placed at mean water surface (P2), correlating with the over-prediction of the peak runup by the qaleFOAM on the bow of the FPSO (see Fig. 6(a) as example).

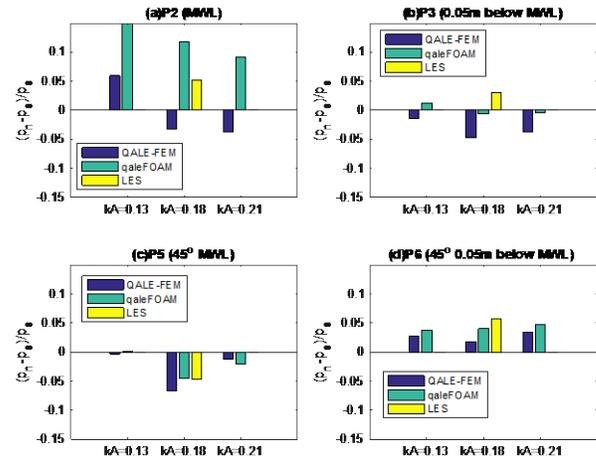


Fig.10 Relative errors of the peak pressure in the cases with different wave steepness

Table 3 Summary of numerical configurations

Case ID	QALE-FEM	qaleFoam	LES
domain size (m)	35×15.5×2.93	35×15.5×2.93(FNPT) 5.4×3×3.53(NS)	6×3×3.3
Duration(s)	30	30	20
CPU	Xeon E3-1545 (2.9G)	Xeon E5-2660 (2.6G)	HPC Wales cluster
cores	4	8	512
parallel	OpenMP	MPI	MPI
No. of Cells	3.45M	1.96M (NS)	16.4M

### Computational time and Efficiency

Further to the analysis of the overall accuracies for different models, the CPU time is also compared in order to assess the robustness of the models. The QALE-FEM simulations are run in a desktop with Intel Xeon E3-1545 (2.9G) using 4 cores with the OpenMP parallelization. The qaleFOAM simulations are run in a workstation with Intel Xeon E5-2660 (2.6GHz) using 8 cores MPI parallel computing. The LES is parallelized using MPI and all cases are run in a 512-core HPC Wales cluster. Tab. 3 and 4 summarises the CPU times spent by different models and the relevant numerical configurations. Considering the

fact that the hardware, especially the number of cores for parallel computing, and the simulation duration used by different models are different. For the purpose of comparison, the CPU effort has been estimated as the total execution time multiplied by the number of cores and divided by the simulation duration, to be consistent with that used by the final report of the blind test (Ransley *et al.*, 2018). It is further scaled by the CPU effort of the QALE-FEM and summarised in Tab. 4. Despite the fact that scaled CPU effort shown in Tab.4 does not reflect the differences of the hardware nor the parallel scalability of different models, one may agree that the QALE-FEM is much faster than other models and the LES is the most time consuming one, attributing to a much finer mesh required to achieve a convergent results (the number of cells in the LES domain is approximately 8 times of that in the NS domain of the qaleFOAM).

Table 4 CPU time spent for achieving convergent results (s)

Case ID	QALE-FEM		qaleFoam		LES	
	Total	Scaled	Total	Scaled	Total	Scaled
11BT1	7780	1	117982	30.3	-	-
12BT1	7986	1	129465	32.4	115200	2769
13BT1	7985	1	144992	36.3	-	-

## CONCLUSIONS

This paper presents a comparative blind numerical test on the interaction between focusing wave and a fixed FPSO-like structure by using the QALE-FEM based on the FNPT model, a two-phase NS model with LES and a hybrid model, qaleFOAM, which coupling the QALE-FEM with OpenFOAM/InterDymFoam. The simulation conditions are specified by the CCP-WSI blind test workshop in ISOPE 2018 and all simulations are carried out independently without the aid of experimental data, which are released after all numerical results have been submitted. It reveals that effectiveness of the wave generation is critical for achieving accurate predictions for the wave-structure interaction problems and the self-correction wavemaker technique delivers promising accuracy for reproducing extreme waves in the numerical wave tank. It also concludes that (1) the overall accuracies of three models in terms of predicting the wave elevation (run-up) and the pressure acting on the FPSO are at a similar level; (2) The QALE-FEM based on the FNPT is more robust than the NS models; the LES is the most time consuming but has better capacity of capturing small-scale physics, especially the turbulence behavior and the vortical structures.

It is important to note that the qaleFOAM and the LES do not result in a better prediction than the QALE-FEM both in terms of the wave elevation/run-up and the pressure, as observed from Fig. 9 and Fig. 10, although they have lower degrees of physical simplification (the viscous and turbulent effects are included in the qaleFOAM and LES models in theory, but they are ignored in the QALE-FEM). Apart from the wave generation problem, the overall accuracy also heavily relies on to what extend the viscous effects and the turbulent effects can be accurately model, e.g. the effectiveness of suppressing the numerical diffusions. This is also worth noting that such conditions are obtained from the cases without wave breaking, aeration and violent wave impact on the FPSO, where the viscous and the turbulent effects are insignificant. Consequently, it may not be applicable to other cases, e.g. those involved slamming impact (Dias and Ghidaglia, 2018).

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